

Two forces  $\mathbf{P}_1$  and  $\mathbf{P}_2$ , of magnitude  $P_1 = 15 \text{ kN}$  and  $P_2 = 18 \text{ kN}$ , are applied as shown to the end  $A$  of bar  $AB$ , which is welded to a cylindrical member  $BD$  of radius  $c = 20 \text{ mm}$  (Fig. 8.21). Knowing that the distance from  $A$  to the axis of member  $BD$  is  $a = 50 \text{ mm}$  and assuming that all stresses remain below the proportional limit of the material, determine (a) the normal and shearing stresses at point  $K$  of the transverse section of member  $BD$  located at a distance  $b = 60 \text{ mm}$  from end  $B$ , (b) the principal axes and principal stresses at  $K$ , (c) the maximum shearing stress at  $K$ .

**Internal Forces in Given Section.** We first replace the forces  $\mathbf{P}_1$  and  $\mathbf{P}_2$  by an equivalent system of forces and couples applied at the center  $C$  of the section containing point  $K$  (Fig. 8.22). This system, which represents the internal forces in the section, consists of the following forces and couples:

1. A centric axial force  $\mathbf{F}$  equal to the force  $\mathbf{P}_1$ , of magnitude
2. A shearing force  $\mathbf{V}$  equal to the force  $\mathbf{P}_2$ , of magnitude
3. A twisting couple  $\mathbf{T}$  of torque  $T$  equal to the moment of  $\mathbf{P}_2$  about the axis of member  $BD$ :

$$F = P_1 = 15 \text{ kN}$$

$$V = P_2 = 18 \text{ kN}$$

4. A bending couple  $\mathbf{M}_y$ , of moment  $M_y$  equal to the moment of  $\mathbf{P}_1$  about a vertical axis through  $C$ :

$$M_y = P_1 a = (15 \text{ kN})(50 \text{ mm}) = 750 \text{ N} \cdot \text{m}$$

5. A bending couple  $\mathbf{M}_z$ , of moment  $M_z$  equal to the moment of  $\mathbf{P}_2$  about a transverse, horizontal axis through  $C$ :

$$M_z = P_2 b = (18 \text{ kN})(60 \text{ mm}) = 1080 \text{ N} \cdot \text{m}$$

The results obtained are shown in Fig. 8.23.

**a. Normal and Shearing Stresses at Point  $K$ .** Each of the forces and couples shown in Fig. 8.23 can produce a normal or shearing stress at point  $K$ . Our purpose is to compute separately each of these stresses, and then to add the normal stresses and add the shearing stresses. But we must first determine the geometric properties of the section.

**Geometric Properties of the Section** We have

$$\begin{aligned} A &= \pi c^2 = \pi(0.020 \text{ m})^2 = 1.257 \times 10^{-3} \text{ m}^2 \\ I_y &= I_z = \frac{1}{4} \pi c^4 = \frac{1}{4} \pi (0.020 \text{ m})^4 = 125.7 \times 10^{-9} \text{ m}^4 \\ J_C &= \frac{1}{2} \pi c^4 = \frac{1}{2} \pi (0.020 \text{ m})^4 = 251.3 \times 10^{-9} \text{ m}^4 \end{aligned}$$

We also determine the first moment  $Q$  and the width  $t$  of the area of the cross section located above the  $z$  axis. Recalling that  $\bar{y} = 4c/3\pi$  for a semicircle of radius  $c$ , we have

$$\begin{aligned} Q &= A \bar{y} = \left( \frac{1}{2} \pi c^2 \right) \left( \frac{4c}{3\pi} \right) = \frac{2}{3} c^3 = \frac{2}{3} (0.020 \text{ m})^3 \\ &= 5.33 \times 10^{-6} \text{ m}^3 \end{aligned}$$

and

$$t = 2c = 2(0.020 \text{ m}) = 0.040 \text{ m}$$

**Normal Stresses.** We observe that normal stresses are produced at  $K$  by the centric force  $\mathbf{F}$  and the bending couple  $\mathbf{M}_y$ , but that the couple  $\mathbf{M}_z$

## EXAMPLE 8.01

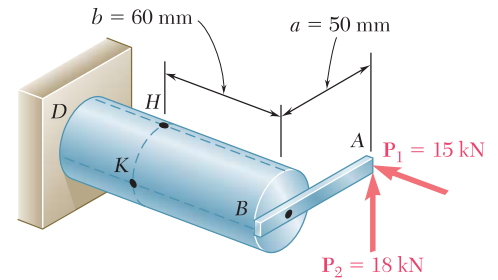


Fig. 8.21

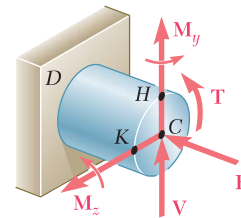


Fig. 8.22

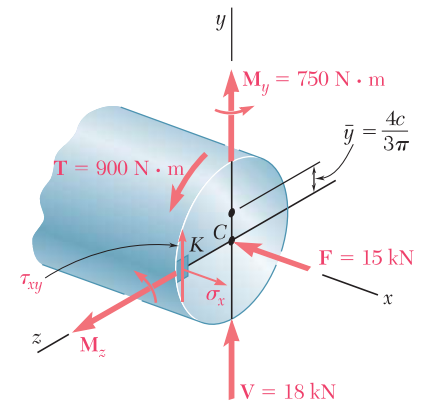


Fig. 8.23

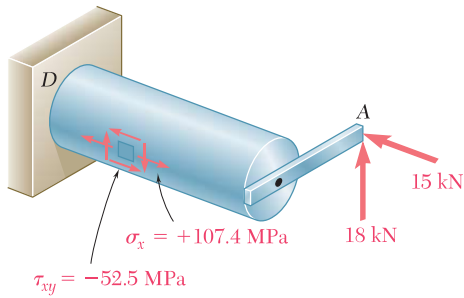


Fig. 8.24

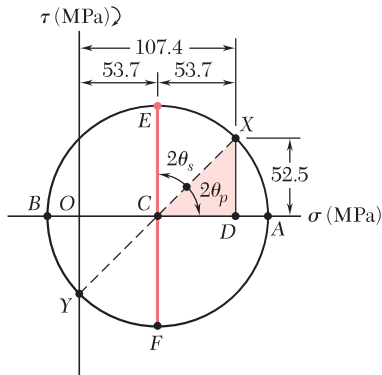


Fig. 8.25

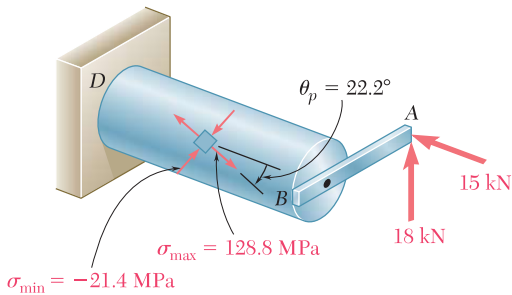


Fig. 8.26

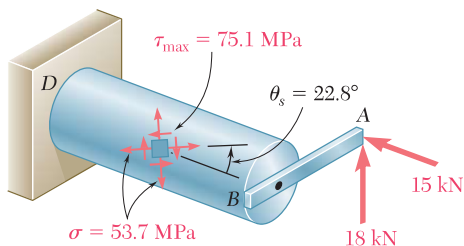


Fig. 8.27

does not produce any stress at  $K$ , since  $K$  is located on the neutral axis corresponding to that couple. Determining each sign from Fig. 8.23, we write

$$\begin{aligned}\sigma_x &= -\frac{F}{A} + \frac{M_y c}{I_y} = -11.9 \text{ MPa} + \frac{(750 \text{ N} \cdot \text{m})(0.020 \text{ m})}{125.7 \times 10^{-9} \text{ m}^4} \\ &= -11.9 \text{ MPa} + 119.3 \text{ MPa} \\ \sigma_x &= +107.4 \text{ MPa}\end{aligned}$$

**Shearing Stresses.** These consist of the shearing stress  $(\tau_{xy})_V$  due to the vertical shear  $\mathbf{V}$  and of the shearing stress  $(\tau_{xy})_{\text{twist}}$  caused by the torque  $\mathbf{T}$ . Recalling the values obtained for  $Q$ ,  $t$ ,  $I_z$ , and  $J_C$ , we write

$$\begin{aligned}(\tau_{xy})_V &= +\frac{VQ}{I_z t} = +\frac{(18 \times 10^3 \text{ N})(5.33 \times 10^{-6} \text{ m}^3)}{(125.7 \times 10^{-9} \text{ m}^4)(0.040 \text{ m})} \\ &= +19.1 \text{ MPa}\end{aligned}$$

$$(\tau_{xy})_{\text{twist}} = -\frac{Tc}{J_C} = -\frac{(900 \text{ N} \cdot \text{m})(0.020 \text{ m})}{251.3 \times 10^{-9} \text{ m}^4} = -71.6 \text{ MPa}$$

Adding these two expressions, we obtain  $\tau_{xy}$  at point  $K$ .

$$\begin{aligned}\tau_{xy} &= (\tau_{xy})_V + (\tau_{xy})_{\text{twist}} = +19.1 \text{ MPa} - 71.6 \text{ MPa} \\ \tau_{xy} &= -52.5 \text{ MPa}\end{aligned}$$

In Fig. 8.24, the normal stress  $\sigma_x$  and the shearing stresses and  $\tau_{xy}$  have been shown acting on a square element located at  $K$  on the surface of the cylindrical member. Note that shearing stresses acting on the longitudinal sides of the element have been included.

**b. Principal Planes and Principal Stresses at Point  $K$ .** We can use either of the two methods of Chap. 7 to determine the principal planes and principal stresses at  $K$ . Selecting Mohr's circle, we plot point  $X$  of coordinates  $\sigma_x = +107.4 \text{ MPa}$  and  $-\tau_{xy} = +52.5 \text{ MPa}$  and point  $Y$  of coordinates  $\sigma_y = 0$  and  $+\tau_{xy} = -52.5 \text{ MPa}$  and draw the circle of diameter  $XY$  (Fig. 8.25). Observing that

$$OC = CD = \frac{1}{2}(107.4) = 53.7 \text{ MPa} \quad DX = 52.5 \text{ MPa}$$

we determine the orientation of the principal planes:

$$\begin{aligned}\tan 2\theta_p &= \frac{DX}{CD} = \frac{52.5}{53.7} = 0.97765 \quad 2\theta_p = 44.4^\circ \downarrow \\ \theta_p &= 22.2^\circ \downarrow\end{aligned}$$

We now determine the radius of the circle,

$$R = \sqrt{(53.7)^2 + (52.5)^2} = 75.1 \text{ MPa}$$

and the principal stresses,

$$\begin{aligned}\sigma_{\max} &= OC + R = 53.7 + 75.1 = 128.8 \text{ MPa} \\ \sigma_{\min} &= OC - R = 53.7 - 75.1 = -21.4 \text{ MPa}\end{aligned}$$

The results obtained are shown in Fig. 8.26.

**c. Maximum Shearing Stress at Point  $K$ .** This stress corresponds to points  $E$  and  $F$  in Fig. 8.25. We have

$$\tau_{\max} = CE = R = 75.1 \text{ MPa}$$

Observing that  $2\theta_s = 90^\circ - 2\theta_p = 90^\circ - 44.4^\circ = 45.6^\circ$ , we conclude that the planes of maximum shearing stress form an angle  $\theta_p = 22.8^\circ \uparrow$  with the horizontal. The corresponding element is shown in Fig. 8.27. Note that the normal stresses acting on this element are represented by  $OC$  in Fig. 8.25 and are thus equal to  $+53.7 \text{ MPa}$ .