

Chapter Four : Pure Bending

Strain = $\frac{\text{change of length}}{\text{original length}}$

$$\epsilon = \frac{\Delta L' - \Delta L}{\Delta L}$$

$$e = \frac{(p-y)\theta - p\theta}{p\theta} = \frac{-y}{p}$$

$$\epsilon = \frac{-y}{p}$$

$$\epsilon_{max} = \frac{-y_{max}}{p} = \frac{+C}{p} \leftarrow \text{absolute}$$

$$\frac{\epsilon}{\epsilon_{max}} = \frac{-y/p}{+C/p} = \frac{y}{C}$$

$$\epsilon = \frac{-y}{C} \epsilon_{max}$$

Hook's Law

$$\sigma = E\epsilon \Rightarrow \left(\epsilon = \frac{\sigma}{E}, \epsilon_{max} = \frac{\sigma_{max}}{E} \right)$$

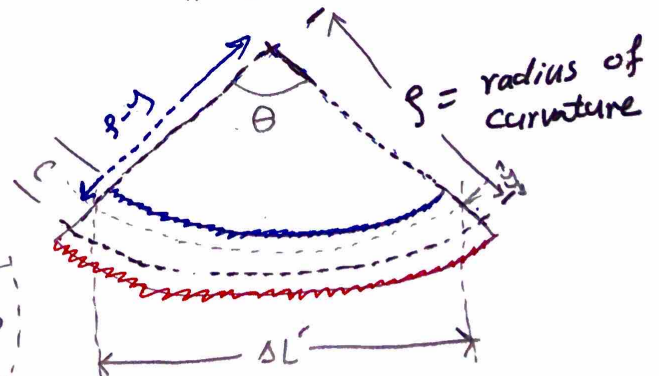
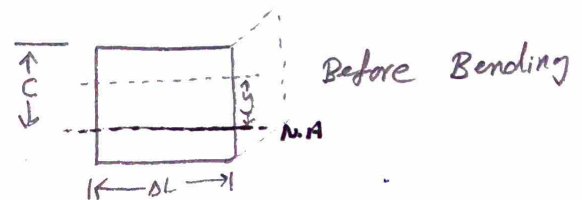
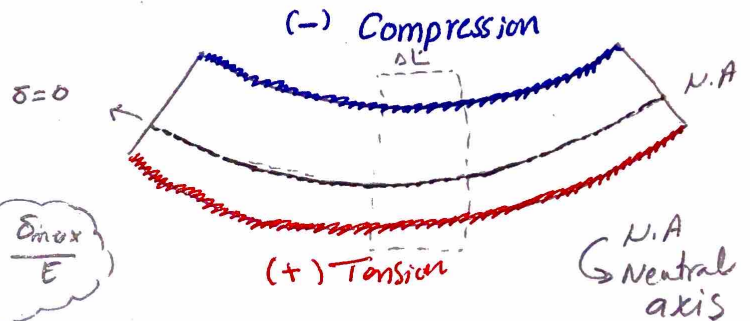
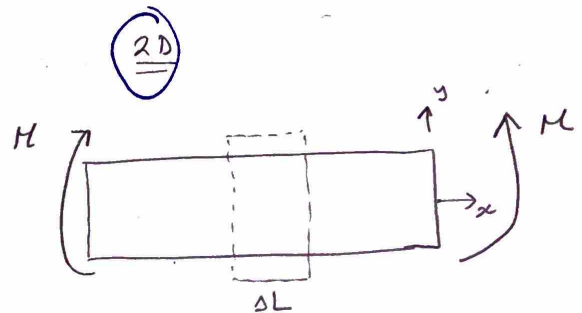
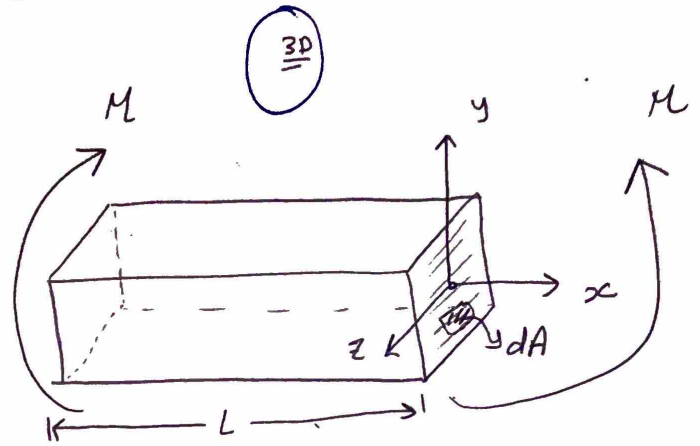
$$\frac{\sigma}{E} = \frac{-y}{C} \cdot \frac{\sigma_{max}}{E}$$

$$\sigma = \frac{-y}{C} \cdot \sigma_{max}$$

$$\sum F = 0 \Rightarrow \int_0^A \sigma dA = 0$$

$$\int \frac{-y}{C} \sigma_{max} dA = 0 \Rightarrow -\frac{\sigma_{max}}{C} \int y dA = 0$$

$\Rightarrow$ only true if N.A is on Centroid (1st moment of area)



cont'd

$$\Sigma M = 0 \Rightarrow M - \int \sigma y dA = 0$$

$$M - \int \frac{-y}{c} \sigma_{\max} y dA = 0$$

$$\Rightarrow M = \frac{-\sigma_{\max}}{c} \left[\int y^2 dA \right] \rightarrow \begin{array}{l} \text{2nd moment of area} \\ \text{moment of Inertia (I)} \end{array}$$

$$\Rightarrow M = \frac{-\sigma_{\max} I}{c}$$

$$\Rightarrow \sigma_{\max} = \frac{-Mc}{I}$$

In general

$$\sigma_{\max} = \frac{-My}{I}$$

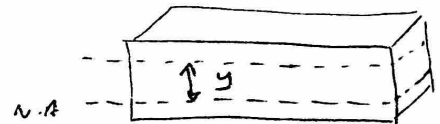
M: Bending moment (N.m)

y: Distance from Centroid (Neutral axis) (m)

I: Moment of Inertia (m⁴)
About Centroidal axis

Hooke's Law $\sigma = E\epsilon$, $\epsilon = \frac{\sigma}{E}$

$$\epsilon = \frac{-My}{EI}$$



also, $\epsilon = \frac{\Delta y}{\rho} = \frac{\Delta y}{EI}$

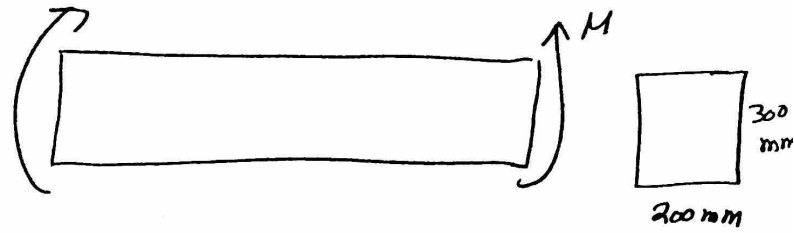
$$\Rightarrow \frac{1}{\rho} = \frac{M}{EI}$$

radius of curvature

Example

$$M = 1000 \text{ N.m}$$

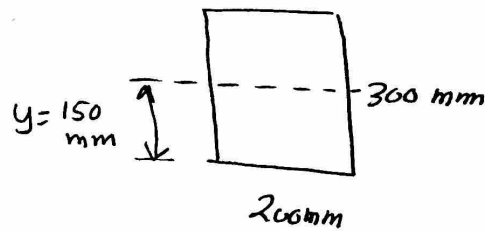
- Find stress at the top surface of the beam

Solution

$$\sigma = \frac{-My}{I}$$

$$= \frac{-(1000)(150)(10^{-3})}{\frac{1}{12} (200 \times 10^{-3}) (300 \times 10^{-3})^3}$$

$$= -0.33 \text{ MPa (compression)}$$



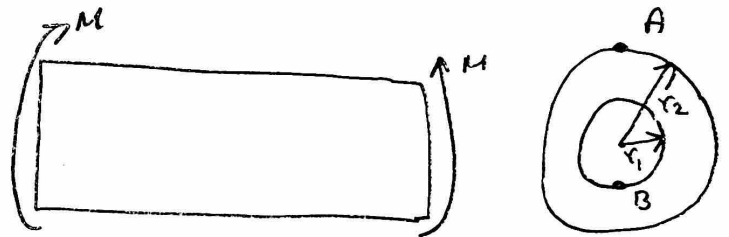
$$I = \frac{1}{12} bh^3$$

Example

$$M = 5 \text{ kN.m}, \quad r_1 = 120 \text{ mm}$$

$$r_2 = 200 \text{ mm}$$

Find stress at A and B

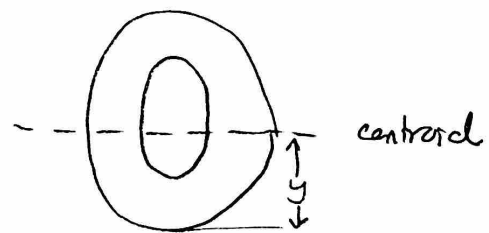
Solution

$$\sigma_A = \frac{-My_A}{I} = \frac{-(5)(10^3)(200)(10^{-3})}{\frac{\pi}{4} [0.20^4 - 0.12^4]}$$

$$\sigma_A = -0.914 \text{ MPa (compression)}$$

$$\sigma_B = \frac{-My_B}{I} = \frac{-(5)(10^3)(120)(10^{-3})}{\frac{\pi}{4} [0.2^4 - 0.12^4]}$$

$$= 0.548 \text{ MPa (Tension)}$$



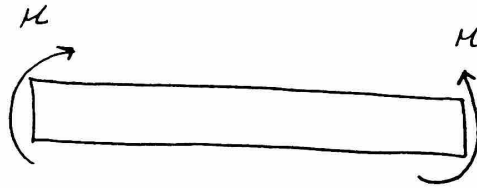
$$I_{\text{circle}} = \frac{\pi}{4} r^4$$

$$I = \frac{\pi}{4} (r_2^4 - r_1^4)$$

Example

$$M = 25 \text{ kNm}$$

Find stress at
C, D and E



Solution

* Centroid

$$\bar{y} = \frac{\sum \bar{y}_i A_i}{\sum A_i} = \frac{\bar{y}_1 A_1 + \bar{y}_2 A_2 + \bar{y}_3 A_3}{A_1 + A_2 + A_3}$$

$$= \frac{(60)(30)(120) + (18)(120)(36) + (60)(30)(120)}{(30)(120) + (120)(36) + (30)(120)}$$

$$= 44.25 \text{ mm}$$

$$I = I_1 + I_2 + I_3$$

$$I = 13.87 \times 10^6 \text{ mm}^4 = 13.87 \times 10^{-6} \text{ m}^4$$

$$\sigma_C = \frac{-M y_C}{I} = \frac{-(25)(10^3)(44.25)(10^{-3})}{13.87 \times 10^{-6}}$$

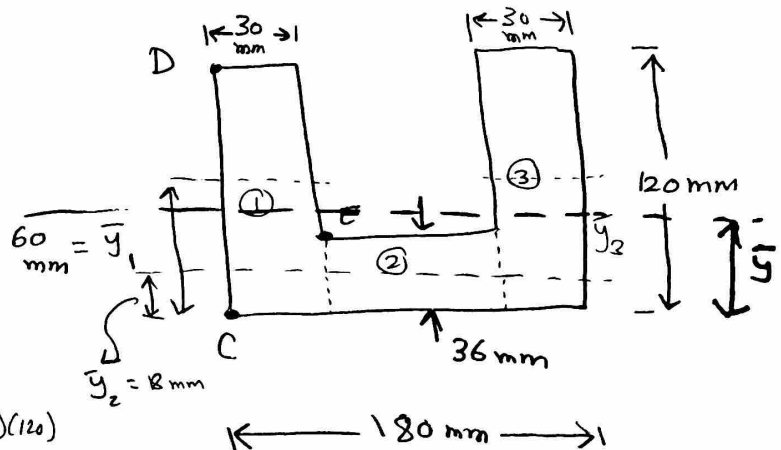
$$\Rightarrow \sigma_C = 79.8 \text{ MPa (tension)}$$

$$\sigma_D = \frac{-M y_D}{I} = \frac{-(25)(10^3)(120 - 44.25)(10^{-3})}{13.87 \times 10^{-6}}$$

$$\sigma_D = -136.5 \text{ MPa (compression)}$$

$$\sigma_E = \frac{-M y_E}{I} = \frac{-(25)(10^3)(36 - 44.25)(10^{-3})}{13.87 \times 10^{-6}}$$

$$\sigma_E = 14.87 \text{ MPa (tension)}$$



Parallel
axis theorem

$$I_1 = \frac{1}{12} b_1 h_1^3 + A_1 d_1^2$$

$$= \frac{1}{12} (30)(120)^3 + (120)(30)(60 - 44.25)^2$$

$$= 5.213 \times 10^6 \text{ mm}^4$$

$$I_3 = I_1$$

$$I_2 = \frac{1}{12} b_2 h_2^3 + A_2 d_2^2$$

$$= \frac{1}{12} (120)(36)^3 + (120)(36)(44.25 - 18)^2$$

$$I_2 = 3.4433 \times 10^6 \text{ mm}^4$$